

6-40	737	0.20	1
7-41	-645	0.56	1
8-42	394	0.08	1
9-43	-328	0.18	1
10-44	205	0.05	1
11-45	282	0.04	1
12-46	-1288	0.34	1
13-47	895	0.11	1
14-48	-3518	0.72	1 4B- 4B
15-49	-422	0.05	1

**UPLIFT REACTION(S) :**

Support	C&C Wind	Main Wind	Non-Wind
1	-439 lb	-626 lb	
2	-439 lb	-626 lb	

  

Type	ID	SECTION	Fy(ksi)	Joints
TC	1	20TC18	50	
BC	1	20TC18	50	
WEB	1	20TC18	50	

20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

**THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.**  
 Loaded for 10 PSF non-concurrent BCLL.  
 Loaded for 200 lb non-concurrent moving BCLL.  
 Galvanization: G60  
 This truss is designed using the ASCE7-16 Wind Specification  
 Bldg Enclosed = Yes,  
 Truss Location = Not End Zone  
 Exp Category = B  
 Bldg Length = 60.00 ft, Bldg Width = 25.00 ft  
 Mean roof height = 11.91 ft, mph = 160  
 Occupancy Category II, Wind Dead Load = 7.20 psf  
 Designed as Main Wind Force Resisting System - Low-rise and Components and Cladding  
 Tributary Area = 64 sqft  
 Uplifts based on elevation at or above 0 ft

This design based on chord bracing applied per the following schedule:

	max o.c.	from	to
TC	12.00"	-2- 0- 0	34- 0- 0
BC	12.00"	0- 0- 0	32- 0- 0

**REACTIONS**

BRG	X-LOC	SIZE	REACT	HORIZ
1	0- 1-12	3.50"	2196	90
2	31-10- 4	3.50"	2195	90

	DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	-0.46" (L/827)	11-12	L/240	81
Vert LL:	-0.31" (L/999)	11-12	L/360	81
Horz TL:	0.15"			
<b>Cantilever</b>				
Vert TL:	-0.11" (L/244)	25-OR	L/ 90	77
Vert LL:	-0.13" (L/201)	25-OR	L/120	77

==== Joint Locations ====

1	0- 0- 0	26	0- 0- 0
2	1- 6- 1	27	0- 2-12
3	1- 7- 3	28	1- 7- 3
4	3- 0-10	29	1- 9- 4
5	3- 2- 6	30	4- 7- 9
6	6- 2- 8	31	4- 9-10
7	6- 4-13	32	7- 9-11
8	9- 4-13	33	8- 0- 0
9	9- 7- 3	34	11- 0- 0
10	12-10- 4	35	11- 2- 6
11	13- 0-10	36	14- 2- 7
12	15- 8-12	37	14- 4-13
13	16- 0- 0	38	17- 4-13
14	16- 1-13	39	17- 7- 3
15	19- 0- 0	40	20- 7- 3
16	19- 2- 6	41	20- 9-10
17	22- 2- 6	42	23- 9-11
18	22- 4-13	43	24- 0- 0
19	25- 4-14	44	27- 0- 5
20	25- 7- 3	45	27- 2- 6
21	28- 7-13	46	30- 1-14
22	28- 9-10	47	30- 3-14
23	30- 3-14	48	31- 9- 5
24	30- 5- 3	49	32- 0- 0
25	32- 0- 0		

== X-Brac. Locations (Joints) ==

BC	TC
34	10
40	16

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL  
 SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL

**NUTRUSSTM**  
 A NUCONSTEEL Product

**WARNING** Read all notes on this sheet and verify all design parameters.  
 Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.  
 When the specified screw count cannot be achieved at the chord to web connections, a 16 gauge gusset plate must be added on both sides of the connection. Typically, gusset plates are at pitch break joints."  
 Min. screw spacing = 9/16" and min. edge distance = 9/16".

<b>Chk:</b>		<b>WO: C11132_Trusses</b>
<b>Dsgnr:</b>		
TC Live	42.00 psf	<b>Design Spec: AISI-2001</b> <b>Buildg Spec: IBC-2018</b> <b>Date: 11/22/2022@</b> <b>Seqn S8.1.0a - 6248</b>
TC Dead	10.00 psf	
BC Live	0.00 psf	
BC Dead	10.00 psf	
<b>TOTAL</b>	<b>62.00 psf</b>	<b>19:53:12</b>