

TC	FORCE	AXL	BND	CSI	ID	SCRWS
OL-1	65	0.01	0.53	0.53	1	
1-2	-70	0.01	0.48	0.49	1	-SP
2-3	-2677	0.34	0.19	0.51	1	SP-
3-4	-2467	0.34	0.16	0.48	1	
4-5	-3198	0.45	0.16	0.59	1	
5-6	-3243	0.45	0.19	0.64	1	
6-7	-3123	0.44	0.22	0.62	1	
7-8	-3045	0.43	0.22	0.65	1	
8-9	-2651	0.31	0.48	0.76	1	
9-10	-2600	0.29	0.55	0.81	1	
10-11	-2069	0.22	0.53	0.73	1	OB- 2B
11-12	-2060	0.29	0.26	0.52	1	2B- OB
12-13	-2572	0.30	0.42	0.70	1	
13-14	-2617	0.31	0.42	0.70	1	
14-15	-3022	0.42	0.24	0.66	1	
15-16	-3102	0.43	0.24	0.63	1	
16-17	-3247	0.45	0.19	0.64	1	
17-18	-3218	0.44	0.18	0.59	1	
18-19	-2521	0.32	0.20	0.49	1	
19-20	-2721	0.35	0.20	0.52	1	-SP
20-21	-69	0.01	0.49	0.50	1	SP-
21-OR	65	0.01	0.53	0.53	1	

UPLIFT REACTION(S) :
 Support C&C Wind Main Wind Non-Wind
 1 -247 lb -187 lb
 2 -247 lb -187 lb

Type ID SECTION Fy (ksi) Joints
 TC 1 20TC18 50
 BC 1 20TC18 50
 WEB 1 20TC18 50

20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 Loaded for 200 lb non-concurrent moving BCLL.
 Galvanization: G60
 This truss is designed using the ASCE7-16 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = End Zone
 Exp Category = B
 Bldg Length = 60.00 ft, Bldg Width = 25.00 ft
 Mean roof height = 12.41 ft, mph = 110
 Occupancy Category II, Wind Dead Load = 7.20 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 52 sqft
 Uplifts based on elevation at or above 0 ft

This design based on chord bracing applied per the following schedule:

	max o.c.	from	to
TC	12.00"	-2-0-0	28-0-0
BC	12.00"	0-0-0	26-0-0

REACTIONS

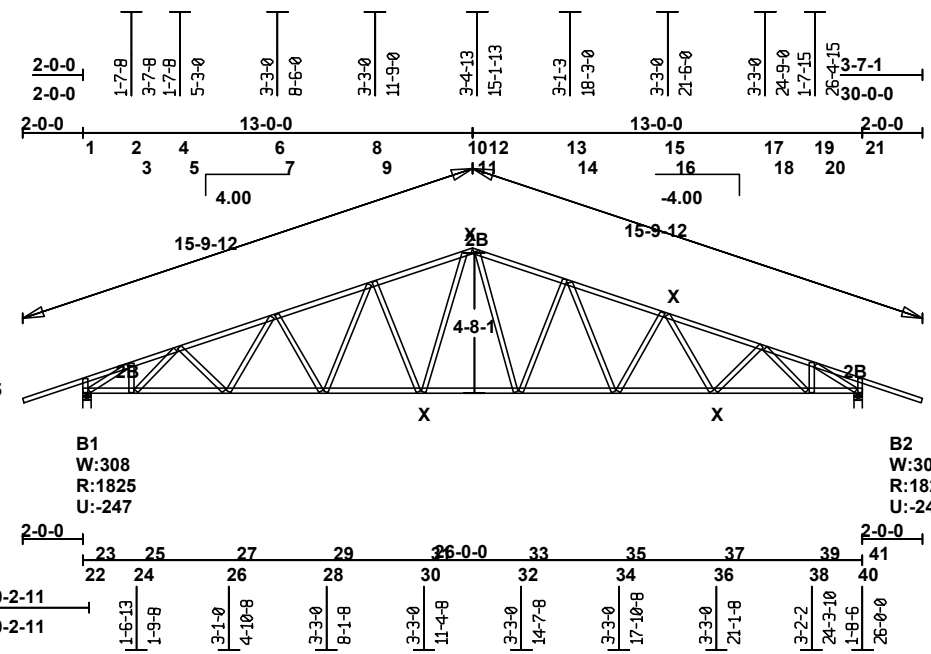
BRG	X-LOC	SIZE	REACT	HORIZ
1	0-1-12	3.50"	1825	59
2	25-10-4	3.50"	1825	59

	DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	-0.31" (L/987)	9-10	L/240	75
Vert LL:	-0.22" (L/999)	9-10	L/360	75
Horz TL:	0.09"			

Cantilever

Vert TL:	-0.12" (L/214)	21-OR	L/90	71
Vert LL:	-0.13" (L/200)	21-OR	L/120	71

BC	FORCE	AXL	BND	CSI	ID	SCRWS
22-23	0	0.00	0.23	0.23	1	-SP
23-24	2343	0.27	0.23	0.50	1	SP-
24-25	2343	0.30	0.16	0.46	1	
25-26	2953	0.36	0.24	0.54	1	
26-27	3031	0.38	0.05	0.43	1	
27-28	3018	0.39	0.09	0.44	1	
28-29	2839	0.36	0.14	0.50	1	
29-30	2668	0.34	0.24	0.55	1	
30-31	2384	0.31	0.22	0.51	1	
31-32	2169	0.28	0.16	0.43	1	
32-33	2368	0.30	0.17	0.46	1	
33-34	2638	0.34	0.18	0.50	1	
34-35	2813	0.36	0.13	0.49	1	
35-36	3005	0.39	0.11	0.45	1	
36-37	3034	0.38	0.05	0.43	1	
37-38	2981	0.37	0.23	0.54	1	
38-39	2392	0.31	0.16	0.46	1	-SP
39-40	2392	0.28	0.23	0.50	1	SP-
40-41	0	0.00	0.23	0.23	1	



==== Joint Locations ====

1	0-0-0	22	0-0-0
2	1-6-6	23	0-2-11
3	1-7-8	24	1-7-8
4	3-1-4	25	1-9-8
5	3-3-0	26	4-8-7
6	6-3-11	27	4-10-8
7	6-6-0	28	7-11-3
8	9-6-10	29	8-1-8
9	9-9-0	30	11-2-2
10	12-8-12	31	11-4-8
11	13-0-0	32	14-5-2
12	13-1-13	33	14-7-8
13	16-0-10	34	17-8-3
14	16-3-0	35	17-10-8
15	19-3-11	36	20-11-7
16	19-6-0	37	21-1-8
17	22-7-4	38	24-1-10
18	22-9-0	39	24-3-10
19	24-3-10	40	25-9-5
20	24-4-15	41	26-0-0
21	26-0-0		

== X-Brac. Locations (Joints) ==

BC	TC
30	10
36	16

WEB	FORCE	CSI	ID	SCRWS
1-22	-428	0.06	1	
2-23	-2780	0.56	1	2B- 2B
3-24	725	0.09	1	
4-25	-1042	0.27	1	
5-26	186	0.02	1	
6-27	231	0.03	1	
7-28	-397	0.21	1	
8-29	494	0.06	1	
9-30	-890	0.77	1	
10-31	881	0.14	1	
12-32	866	0.14	1	
13-33	-859	0.76	1	
14-34	520	0.07	1	
15-35	-437	0.24	1	
16-36	243	0.03	1	
17-37	166	0.02	1	
18-38	-1001	0.27	1	
19-39	692	0.09	1	
20-40	-2820	0.59	1	2B- 2B
21-41	-423	0.05	1	

B1
 W:308
 R:1825
 U:-247

B2
 W:308
 R:1825
 U:-247

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL
 SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL

Scale: 5/32" = 1'



WARNING Read all notes on this sheet and verify all design parameters.
 Truss design on this sheet is only valid with NUTRUS sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.

Designer:		WO: C11026_Trusses
Dsgn Chk:		
Engg Chk:		
Cutting :		
TC Live	42.00 psf	Design Spec: AISI S100-2012
TC Dead	10.00 psf	Buildg Spec: IBC-2018
BC Live	0.00 psf	
BC Dead	10.00 psf	
TOTAL	62.00 psf	Date: 11/22/2022@ 18:31:3
		Seqn S8.1.0a - 6222