

9-23	-380	0.69	1
10-24	-460	0.54	1
11-25	-193	0.22	1
12-26	-426	0.27	1
13-27	-246	0.16	1
14-28	108	0.03	1
15-29	-466	0.09	1

UPLIFT REACTION(S) :

Support	C&C Wind	Main Wind	Non-Wind
1	-302 lb	-326 lb	
3	-34 lb	-31 lb	
6	-180 lb	-187 lb	-35 lb
7		-29 lb	-115 lb
10	-307 lb	-215 lb	-53 lb
11	-349 lb	-193 lb	
14	-190 lb	-94 lb	-67 lb
15	-311 lb	-210 lb	-48 lb
18	-7 lb	-48 lb	-83 lb
19	-170 lb	-182 lb	-73 lb
22	-74 lb	-51 lb	
24	-298 lb	-319 lb	

Type	ID	SECTION	Fy(ksi)	Joints
TC	1	20TC20	50	
BC	1	20TC20	50	
WEB	1	20TC20	50	

20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 Loaded for 200 lb non-concurrent moving BCLL.
 Mark all interior bearing locations.
 Install interior support(s) before erection.
 This truss is designed using the ASCE7-16 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = End Zone
 Exp Category = B
 Bldg Length = 60.00 ft, Bldg Width = 25.00 ft
 Mean roof height = 12.41 ft, mph = 160
 Occupancy Category II, Wind Dead Load = 7.20 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 52 sqft
 Uplifts based on elevation at or above 0 ft

This design based on chord bracing applied per the following schedule:

	max o.c.	from	to
TC	12.00"	-2- 0- 0	28- 0- 0
BC	12.00"	0- 0- 0	26- 0- 0

Galvanization: G60

REACTIONS

Brg	Reac	Horiz	Brg	Reac	Horiz
1	482	0	13	210	0
2	151	0	14	368	-140
3	212	184	15	409	238
4	212	-8	16	209	0
5	187	0	17	184	0
6	307	-330	18	259	-102
7	190	88	19	300	318
8	181	0	20	178	0
9	197	0	21	210	0
10	414	-245	22	231	-227
11	538	217	23	164	9
12	200	0	24	475	0

DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	-0.09" (L/999)	9-10	L/240 40
Vert LL:	-0.07" (L/999)	9-10	L/360 40
Horz TL:	0.01"		

Cantilever

Vert TL:	-0.13" (L/186)	OL-1	L/ 90 1
Vert LL:	-0.10" (L/230)	OL-1	L/120 1

==== Joint Locations ====

1	0- 0- 0	16	0- 0- 0
2	0- 2- 5	17	2- 1- 7
3	4- 2- 6	18	2- 2- 0
4	4- 4- 0	19	6- 4- 0
5	8- 5-12	20	6- 6- 0
6	8- 8- 0	21	10- 7-11
7	12- 8-12	22	10-10- 0
8	13- 0- 0	23	14-11-11
9	13- 1-13	24	15- 2- 0
10	17- 1-12	25	19- 4- 0
11	17- 4- 0	26	19- 6- 0
12	21- 6- 6	27	23- 9- 7
13	21- 8- 0	28	23-10- 0
14	25- 9-11	29	26- 0- 0
15	26- 0- 0		

== X-Brac. Locations (Joints) ==

BC	TC
21	7
27	13

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL
 SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL

NUTRUSSTM
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WARNING Read all notes on this sheet and verify all design parameters.
 Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.
 When the specified screw count cannot be achieved at the chord to web connections, a 16 gauge gusset plate must be added on both sides of the connection. Typically, gusset plates are at pitch break joints."
 Min. screw spacing = 9/16" and min. edge distance = 9/16".

Chk:		WO: C11126_Trusses
Dsgnr:		
TC Live	42.00 psf	Design Spec: AISI-2001
TC Dead	10.00 psf	Buildg Spec: IBC-2018
BC Live	0.00 psf	
BC Dead	10.00 psf	
TOTAL	62.00 psf	Date: 11/23/2022@ 17:15:40
		Seqn S8.1.0a - 6285