

UPLIFT REACTION(S) :			
Support	C&C Wind	Main Wind	Non-Wind
1	-260 lb	-295 lb	
3	-52 lb	-54 lb	
6	-188 lb	-209 lb	-71 lb
7	-156 lb	-163 lb	-112 lb
10	-64 lb	-79 lb	
11	-195 lb	-125 lb	-127 lb
12	-119 lb	-53 lb	
15	-32 lb		
16	-203 lb	-125 lb	-128 lb
17	-53 lb	-71 lb	
20	-173 lb	-176 lb	-89 lb
21	-185 lb	-206 lb	-103 lb
24	-76 lb	-68 lb	
26	-255 lb	-289 lb	

Type	ID	SECTION	Fy (ksi)	Joints
TC	1	20TC20	50	
BC	1	20TC20	50	
WEB	1	20TC20	50	

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loaded for 10 PSF non-concurrent BCLL.
 Loaded for 200 lb non-concurrent moving BCLL.
 Mark all interior bearing locations.
 Install interior support(s) before erection.
 This truss is designed using the ASCE7-16 Wind Specification
 Bldg Enclosed = Yes,
 Truss Location = End Zone
 Exp Category = B
 Bldg Length = 60.00 ft, Bldg Width = 25.00 ft
 Mean roof height = 12.75 ft, mph = 160
 Occupancy Category II, Wind Dead Load = 7.20 psf
 Designed as Main Wind Force Resisting System
 - Low-rise and Components and Cladding
 Tributary Area = 60 sqft
 Uplifts based on elevation at or above 0 ft
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

This design based on chord bracing applied per the following schedule:

	max o.c.	from	to
TC	12.00"	-2- 0- 0	32- 0- 0
BC	12.00"	0- 0- 0	30- 0- 0

Galvanization: G60

REACTIONS

Brg	Reac	Horiz	Brg	Reac	Horiz
1	447	0	14	216	0
2	192	0	15	320	-50
3	249	169	16	240	0
4	201	-10	17	307	59
5	216	0	18	210	0
6	388	-364	19	209	0
7	478	298	20	508	-316
8	193	0	21	349	363
9	217	0	22	210	0
10	344	-69	23	211	0
11	241	0	24	255	-186
12	283	89	25	197	15
13	203	0	26	438	0

	DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	-0.14" (L/999)	13-14	L/240	40
Vert LL:	-0.12" (L/999)	13-14	L/360	40
Horz TL:	0.00"			

Cantilever

Vert TL:	-0.09" (L/271)	OL-1	L/ 90	1
Vert LL:	-0.07" (L/335)	OL-1	L/120	1

==== Joint Locations =====

1	0- 0- 0	18	0- 0- 0
2	0- 2- 4	19	2- 5-10
3	4-10- 6	20	2- 6- 0
4	5- 0- 0	21	7- 4- 1
5	9- 9-12	22	7- 6- 0
6	10- 0- 0	23	12- 1- 8
7	12- 3-11	24	12- 3-11
8	14- 8-12	25	12- 6- 0
9	15- 0- 0	26	17- 3-11
10	15- 1-13	27	17- 6- 0
11	17- 6- 0	28	17- 8- 3
12	19- 9-12	29	22- 4- 0
13	20- 0- 0	30	22- 6- 0
14	24-10- 7	31	27- 5-10
15	25- 0- 0	32	27- 6- 0
16	29- 9-12	33	30- 0- 0
17	30- 0- 0		

== X-Brac. Locations (Joints) ==

BC	TC
23	7
29	13

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL
 SCRWs = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL

NUTRUSSTM
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WARNING Read all notes on this sheet and verify all design parameters.
 Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.
 When the specified screw count cannot be achieved at the chord to web connections, a 16 gauge gusset plate must be added on both sides of the connection. Typically, gusset plates are at pitch break joints."
 Min. screw spacing = 9/16" and min. edge distance = 9/16".

Chk:		WO: C11130_Trusses	
Dsgnr:		Design Spec:	AISI-2001
TC Live	42.00 psf	Buildg Spec:	IBC-2018
TC Dead	10.00 psf	Date:	11/24/2022@
BC Live	0.00 psf	Seqn	S8.1.0a - 6350
BC Dead	10.00 psf		
TOTAL	62.00 psf		