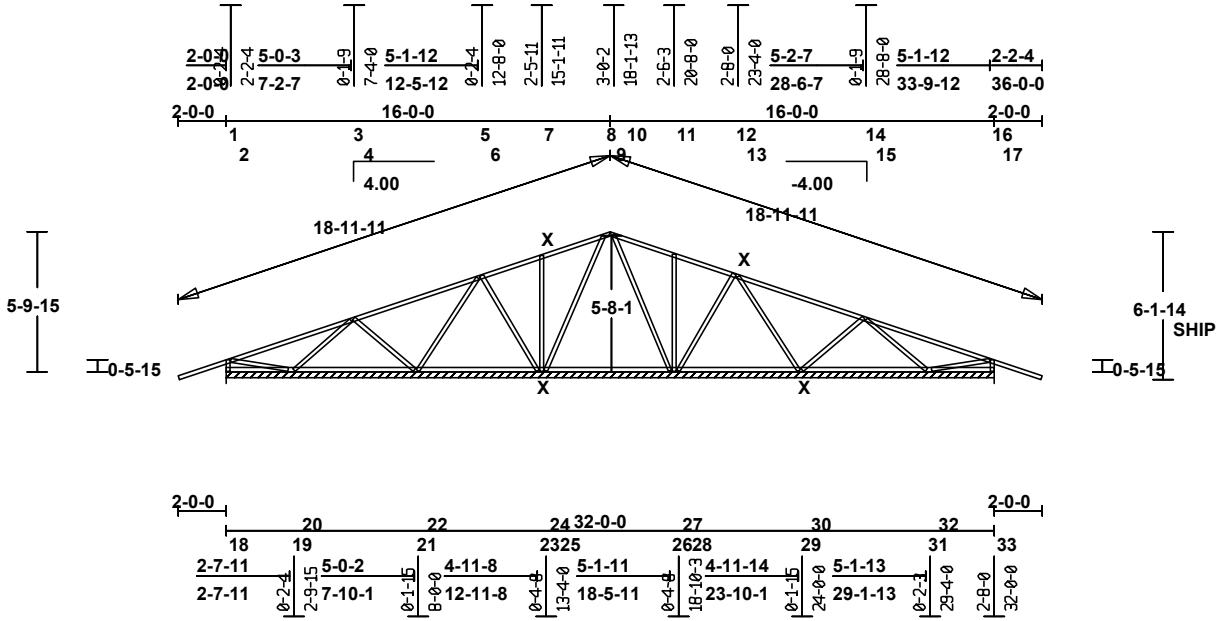
Job Name:	Truss ID: C11132	Qty: 1	SPACING: 2- 0- 0	PLY: 1	WEIGHT: 103.42

BRG	X-LOC	REACT	SIZE
1	0- 1-12	423	3.50"
2	1- 3-13	201	3.50"
3	2- 7-11	208	3.50"
4	2- 9-15	402	3.50"
5	4- 5-15	221	3.50"
6	6- 2- 0	203	3.50"
7	7-10- 1	399	3.50"
8	8- 0- 0	498	3.50"
9	9- 7-13	215	3.50"
10	11- 3-11	213	3.50"
11	12-11- 8	370	3.50"
12	13- 1-11	261	3.50"
13	13- 4- 0	307	3.50"
14	15- 0- 9	204	3.50"
15	16- 9- 2	218	3.50"
16	18- 5-11	312	3.50"
17	18- 8- 0	259	3.50"
18	18-10- 3	369	3.50"
19	20- 6- 2	221	3.50"
20	22- 2- 2	203	3.50"
21	23-10- 1	533	3.50"
22	24- 0- 0	446	3.50"
23	25- 8-10	214	3.50"
24	27- 5- 3	221	3.50"
25	29- 1-13	426	3.50"
26	29- 4- 0	220	3.50"
27	30- 8- 0	203	3.50"
28	31-10- 4	413	3.50"

TC	FORCE	AXL	BND	CSI	ID	SCRWS
OL-1	65	0.01	0.67	0.67	1	
1-2	-65	0.01	0.61	0.62	1	
2-3	214	0.04	0.71	0.72	1	
3-4	-116	0.02	0.77	0.79	1	
4-5	383	0.02	0.77	0.77	1	
5-6	-82	0.01	0.57	0.58	1	
6-7	72	0.01	0.72	0.73	1	
7-8	83	0.01	0.23	0.23	1	
8-9	-94	0.02	0.03	0.04	1	
9-10	-69	0.01	0.02	0.04	1	
10-11	-98	0.00	0.24	0.24	1	
11-12	-103	0.02	0.73	0.75	1	
12-13	-127	0.02	0.59	0.61	1	
13-14	367	0.02	0.80	0.80	1	
14-15	-134	0.03	0.80	0.82	1	
15-16	250	0.04	0.75	0.76	1	
16-17	-62	0.01	0.61	0.62	1	
17-OR	65	0.01	0.67	0.67	1	

BC	FORCE	AXL	BND	CSI	ID	SCRWS
18-19	0	0.00	0.10	0.10	1	
19-20	0	0.00	0.13	0.13	1	
20-21	0	0.00	0.13	0.13	1	
21-22	0	0.00	0.12	0.12	1	
22-23	0	0.00	0.12	0.12	1	
23-24	0	0.00	0.11	0.12	1	
24-25	0	0.00	0.10	0.10	1	
25-26	0	0.00	0.12	0.12	1	
26-27	0	0.00	0.12	0.12	1	
27-28	0	0.00	0.13	0.13	1	
28-29	0	0.00	0.13	0.13	1	
29-30	0	0.00	0.13	0.13	1	
30-31	0	0.00	0.14	0.14	1	
31-32	0	0.00	0.14	0.14	1	
32-33	0	0.00	0.10	0.10	1	



OVER CONTINUOUS SUPPORT

Scale: 1/8" = 1'

NUTRUSS™
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WARNING *Read all notes on this sheet and verify all design parameters.*

Truss design on this sheet is only valid with NUTRUSS sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.

Designer:	
Dsgn Chk:	
Engg Chk:	
Cutting :	
TC Live	42.00 psf
TC Dead	10.00 psf
BC Live	0.00 psf
BC Dead	10.00 psf
TOTAL	62.00 psf

WO: C11132_Trusses

Design Spec: AISI S100-2012
Buildg Spec: IBC-2018

Date: 11/24/2022@ 16:00:50
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Job Name: | Truss ID: C11132 | Qty: 1 | SPACING: 2- 0- 0 | PLY: 1 | WEIGHT: 103.42

WEB	FORCE	CSI	ID	SCRWS
1-18	-412	0.08	1	
2-19	-122	0.06	1	
3-20	-357	0.29	1	
4-21	-480	0.43	1	
5-22	-573	0.90	1	
6-23	-145	0.22	1	
7-24	-289	0.48	1	
8-25	-247	0.61	1	
10-26	-143	0.35	1	
11-27	-288	0.49	1	
12-28	-125	0.20	1	
13-29	-615	0.99	1	
14-30	-473	0.43	1	
15-31	-414	0.36	1	
16-32	-154	0.08	1	
17-33	-402	0.07	1	

UPLIFT REACTION(S) :

Support	C&C Wind	Main Wind	Non-Wind
1	-109 lb	-106 lb	
3	-6 lb	-6 lb	-187 lb
4		-4 lb	
7	-44 lb	-52 lb	-122 lb
8	-14 lb	-30 lb	-51 lb
11	-7 lb	-15 lb	-15 lb
12	-47 lb	-37 lb	-124 lb
13	-19 lb	-2 lb	
17	-50 lb	-37 lb	-146 lb
18	-5 lb	-12 lb	-9 lb
21	-18 lb	-35 lb	-107 lb
22	-42 lb	-51 lb	-71 lb
25	-3 lb	-6 lb	
26	-7 lb	-6 lb	-197 lb
28	-108 lb	-104 lb	

Type	ID	SECTION	Fy(ksi)	Joints
TC	1	20TC20	50	
BC	1	20TC20	50	
WEB	1	20TC20	50	

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
Loaded for 10 PSF non-concurrent BCLL.
Loaded for 200 lb non-concurrent moving BCLL.
Mark all interior bearing locations.
Install interior support(s) before erection.
This truss is designed using the ASCE7-16 Wind Specification
Bldg Enclosed = Yes,
Truss Location = Not End Zone
Exp Category = B
Bldg Length = 60.00 ft, Bldg Width = 25.00 ft
Mean roof height = 11.91 ft, mph = 110
Occupancy Category II, Wind Dead Load = 7.20 psf
Designed as Main Wind Force Resisting System
- Low-rise and Components and Cladding
Tributary Area = 64 sqft
Uplifts based on elevation at or above 0 ft
20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

This design based on chord bracing applied per the following schedule:

	max o.c.	from	to
TC	12.00"	-2- 0- 0	34- 0- 0
BC	12.00"	0- 0- 0	32- 0- 0

Galvanization: G60

REACTIONS

Brg	Reac	Horiz	Brg	Reac	Horiz
1	423	0	15	218	0
2	201	0	16	312	-55
3	208	-120	17	259	0
4	402	269	18	369	63
5	221	0	19	221	0
6	203	0	20	203	0
7	399	-368	21	533	-333
8	498	312	22	446	361
9	215	0	23	214	0
10	213	0	24	221	0
11	370	-74	25	426	-316
12	261	0	26	220	152
13	307	98	27	203	0
14	204	0	28	413	0

	DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	-0.16" (L/999)	13-14	L/240	40
Vert LL:	-0.14" (L/999)	13-14	L/360	40
Horz TL:	0.01"			
Cantilever				
Vert TL:	-0.06" (L/407)	OL-1	L/ 90	41
Vert LL:	-0.05" (L/504)	OL-1	L/120	41

===== Joint Locations =====

1	0- 0- 0	18	0- 0- 0
2	0- 2- 4	19	2- 7-11
3	5- 2- 7	20	2- 9-15
4	5- 4- 0	21	7-10- 1
5	10- 5-12	22	8- 0- 0
6	10- 8- 0	23	12-11- 8
7	13- 1-11	24	13- 1-11
8	15- 8-12	25	13- 4- 0
9	16- 0- 0	26	18- 5-11
10	16- 1-13	27	18- 8- 0
11	18- 8- 0	28	18-10- 3
12	21- 1-12	29	23-10- 1
13	21- 4- 0	30	24- 0- 0
14	26- 6- 7	31	29- 1-13
15	26- 8- 0	32	29- 4- 0
16	31- 9-12	33	32- 0- 0
17	32- 0- 0		

== X-Brac. Locations (Joints)==

BC	TC
23	7
29	13

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL
SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL

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When the specified screw count cannot be achieved at the chord to web connections, a 16 gauge gusset plate must be added on both sides of the connection. Typically, gusset plates are at pitch break joints."
Min. screw spacing = 9/16" and min. edge distance = 9/16".

Chk:
Dsgnr:

TC Live	42.00 psf
TC Dead	10.00 psf
BC Live	0.00 psf
BC Dead	10.00 psf
TOTAL	62.00 psf

WO: C11132_Trusses

Design Spec: AISI-2001
Buildg Spec: IBC-2018

Date: 11/24/2022@ 16:00:50
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