

WEB	FORCE	CSI ID	SCRWS	UPLIFT	REACTION(S)
				Support	C&C Wind Main Wind Non-Wind
1-18	-412	0.08	1	1	-275 lb -268 lb
2-19	-129	0.07	1	3	-17 lb -16 lb -187 lb
3-20	-357	0.29	1	4	-30 lb -40 lb
4-21	-480	0.43	1	7	-130 lb -147 lb -122 lb
5-22	-573	0.90	1	8	-75 lb -110 lb -51 lb
6-23	-145	0.22	1	11	-35 lb -51 lb -15 lb
7-24	-289	0.48	1	12	-115 lb -93 lb -124 lb
8-25	-247	0.61	1	13	-69 lb -32 lb
10-26	-143	0.35	1	16	-15 lb
11-27	-288	0.49	1	17	-120 lb -94 lb -146 lb
12-28	-125	0.20	1	18	-29 lb -44 lb -9 lb
13-29	-615	0.99	1	21	-86 lb -122 lb -107 lb
14-30	-473	0.43	1	22	-126 lb -143 lb -71 lb
15-31	-414	0.36	1	25	-41 lb -48 lb
16-32	-154	0.08	1	26	-18 lb -17 lb -197 lb
17-33	-402	0.07	1	28	-272 lb -263 lb

Type	ID	SECTION	Fy (ksi)	Joints
TC	1	20TC20	50	
BC	1	20TC20	50	
WEB	1	20TC20	50	

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 Loaded for 10 PSF non-concurrent BCLL.  
 Loaded for 200 lb non-concurrent moving BCLL.  
 Mark all interior bearing locations.  
 Install interior support(s) before erection.  
 This truss is designed using the ASCE7-16 Wind Specification  
 Bldg Enclosed = Yes,  
 Truss Location = Not End Zone  
 Exp Category = B  
 Bldg Length = 60.00 ft, Bldg Width = 25.00 ft  
 Mean roof height = 11.91 ft, mph = 160  
 Occupancy Category II, Wind Dead Load = 7.20 psf  
 Designed as Main Wind Force Resisting System  
 - Low-rise and Components and Cladding  
 Tributary Area = 64 sqft  
 Uplifts based on elevation at or above 0 ft  
 20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

This design based on chord bracing applied per the following schedule:

	max o.c.	from	to
TC	12.00"	-2-0-0	34-0-0
BC	12.00"	0-0-0	32-0-0

Galvanization: G60

REACTIONS

Brg	Reac	Horiz	Brg	Reac	Horiz
1	423	0	15	218	0
2	201	0	16	312	-55
3	208	-128	17	259	0
4	402	269	18	369	63
5	221	0	19	221	0
6	203	0	20	203	0
7	399	-368	21	533	-333
8	498	312	22	446	361
9	215	0	23	214	0
10	213	0	24	221	0
11	370	-74	25	426	-316
12	261	0	26	220	152
13	307	98	27	203	0
14	204	0	28	413	0

DEFLECTION LOC. ALLOW. LC

Vert TL:	(L/999)	13-14	L/240	40
Vert LL:	(L/999)	13-14	L/360	40
Horz TL:	0.01"			

Cantilever

Vert TL:	(L/407)	OL-1	L/90	41
Vert LL:	(L/504)	OL-1	L/120	41


==== Joint Locations ====

1	0-0-0	18	0-0-0
2	0-2-4	19	2-7-11
3	5-2-7	20	2-9-15
4	5-4-0	21	7-10-1
5	10-5-12	22	8-0-0
6	10-8-0	23	12-11-8
7	13-1-11	24	13-1-11
8	15-8-12	25	13-4-0
9	16-0-0	26	18-5-11
10	16-1-13	27	18-8-0
11	18-8-0	28	18-10-3
12	21-1-12	29	23-10-1
13	21-4-0	30	24-0-0
14	26-6-7	31	29-1-13
15	26-8-0	32	29-4-0
16	31-9-12	33	32-0-0
17	32-0-0		

== X-Brac. Locations (Joints) ==

BC	TC
23	7
29	13

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL  
 SCRWS = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL



**WARNING** Read all notes on this sheet and verify all design parameters.

Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.

*When the specified screw count cannot be achieved at the chord to web connections, a 16 gauge gusset plate must be added on both sides of the connection. Typically, gusset plates are at pitch break joints.*

*Min. screw spacing = 9/16" and min. edge distance = 9/16".*

Chk:		WO: C11132_Trusses
Dsgnr:		
TC Live	42.00 psf	Design Spec: AISI-2001
TC Dead	10.00 psf	Buildg Spec: IBC-2018
BC Live	0.00 psf	
BC Dead	10.00 psf	
TOTAL	62.00 psf	Date: 11/24/2022@ 16:02:06
		Seqn S8.1.0a - 6357