

2-19	236	0.06	1
3-20	-227	0.24	1
4-21	-399	0.43	1
5-22	-465	0.38	1
6-23	-140	0.32	1
7-24	-313	0.85	1
8-25	-230	0.84	1
10-26	-163	0.59	1
11-27	-303	0.84	1
12-28	-131	0.30	1
13-29	-494	0.41	1
14-30	-381	0.43	1
15-31	-300	0.34	1
16-32	268	0.07	1
17-33	-394	0.07	1

UPLIFT REACTION(S) :

Support	C&C Wind	Main Wind	Non-Wind
1	-299 lb	-301 lb	
3	-9 lb	-24 lb	
6	-292 lb	-277 lb	-55 lb
7	-200 lb	-157 lb	-90 lb
10	-95 lb	-99 lb	
11	-233 lb	-148 lb	-125 lb
12	-81 lb	-23 lb	
16	-234 lb	-145 lb	-127 lb
17	-88 lb	-94 lb	
20	-211 lb	-166 lb	-67 lb
21	-284 lb	-270 lb	-87 lb
24	-57 lb	-45 lb	
26	-282 lb	-285 lb	

Type	ID	SECTION	Fy(ksi)	Joints
TC	1	20TC20	50	
BC	1	20TC20	50	
WEB	1	20TC20	50	

20 psf bottom chord live load NOT required on this truss, per IBC/IRC requirements for attics with limited storage.

THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.  
 Loaded for 10 PSF non-concurrent BCLL.  
 Loaded for 200 lb non-concurrent moving BCLL.  
 Mark all interior bearing locations.  
 Install interior support(s) before erection.  
 This truss is designed using the ASCE7-16 Wind Specification  
 Bldg Enclosed = Yes,  
 Truss Location = End Zone  
 Exp Category = B  
 Bldg Length = 60.00 ft, Bldg Width = 25.00 ft  
 Mean roof height = 14.00 ft, mph = 160  
 Occupancy Category II, Wind Dead Load = 7.20 psf  
 Designed as Main Wind Force Resisting System  
 - Low-rise and Components and Cladding  
 Tributary Area = 60 sqft  
 Uplifts based on elevation at or above 0 ft

This design based on chord bracing applied per the following schedule:

	max o.c.	from	to
TC	12.00"	-2- 0- 0	32- 0- 0
BC	12.00"	0- 0- 0	30- 0- 0

Galvanization: G60

REACTIONS

Brg	Reac	Horiz	Brg	Reac	Horiz
1	427	0	14	216	0
2	189	0	15	323	-43
3	249	265	16	279	0
4	199	-9	17	302	48
5	217	0	18	210	0
6	374	-253	19	209	0
7	440	188	20	467	-197
8	192	0	21	326	239
9	217	0	22	209	0
10	339	-52	23	211	0
11	290	0	24	275	-254
12	279	65	25	198	15
13	203	0	26	404	0

	DEFLECTION	LOC.	ALLOW.	LC
Vert TL:	-0.11" (L/999)	13-14	L/240	40
Vert LL:	-0.09" (L/999)	13-14	L/360	40
Horz TL:	0.01"			

Cantilever

Vert TL:	-0.09" (L/251)	OL-1	L/ 90	1
Vert LL:	-0.08" (L/311)	OL-1	L/120	1

==== Joint Locations ====

1	0- 0- 0	18	0- 0- 0
2	0- 2- 7	19	2- 5- 5
3	4-10- 0	20	2- 6- 0
4	5- 0- 0	21	7- 3-13
5	9- 9-10	22	7- 6- 0
6	10- 0- 0	23	12- 1- 6
7	12- 3-10	24	12- 3-10
8	14- 9- 1	25	12- 6- 0
9	15- 0- 0	26	17- 3-10
10	15- 2- 1	27	17- 6- 0
11	17- 6- 0	28	17- 8- 4
12	19- 9-10	29	22- 3-12
13	20- 0- 0	30	22- 6- 0
14	24-10- 0	31	27- 5- 6
15	25- 0- 0	32	27- 6- 0
16	29- 9- 9	33	30- 0- 0
17	30- 0- 0		

== X-Brac. Locations (Joints) ==

BC	TC
23	7
29	13

Each connection requires 3/8" diameter proprietary bolt supplied by NUCONSTEEL  
 SCRWs = The required number of double-sided #14 screws at each end of the truss member: SP = Spacer supplied by NUCONSTEEL

**NUTRUSSTM**  
 A NUCONSTEEL Product

**WARNING** Read all notes on this sheet and verify all design parameters.  
 Truss design on this sheet is only valid with NUTRUSST sections and is for an individual building component, not a truss system. Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is part of the building design and which must be considered by the building designer. Bracing shown is lateral bracing of truss members only. Any additional bracing, temporary and/or permanent, is the responsibility of the truss erector and/or the building designer. The Professional Engineer's seal indicates only that the truss assembly shown on this sheet meets the acceptable design criteria for the loads, loading condition, truss configuration and spans specified.  
 When the specified screw count cannot be achieved at the chord to web connections, a 16 gauge gusset plate must be added on both sides of the connection. Typically, gusset plates are at pitch break joints."  
 Min. screw spacing = 9/16" and min. edge distance = 9/16".

Chk:	Dsgnr:	WO: C61230E_Trusses
TC Live	42.00 psf	Design Spec: AISI-2001
TC Dead	10.00 psf	Buildg Spec: IBC-2018
BC Live	0.00 psf	Date: 11/24/2022@
BC Dead	10.00 psf	Seqn S8.1.0a - 6321
<b>TOTAL</b>	<b>62.00 psf</b>	12:16:42